

DIGITAL IMAGE CORRELATION SYSTEMS IN THE EXPERIMENTAL INVESTIGATIONS: CAPABILITIES AND LIMITATIONS

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The aim of the paper is to present the possibilities and limitations of using the Digital Image Correlation systems. In order to assess the measurement inaccuracies the measuring volume 1250×1100 mm was analysed using two cameras with sensor resolution 6 megapixels. It was stated very good accuracy of the line segment length change. It causes that observation of crack widths can be considered as precisely. Some practical information concern how determine the compatibility between crack width measured traditionally and by using DIC are given. In the second part of the paper the results of the tests concerning capacity of interface between two concrete casting at the same time were presented. Use of the optical measurement system Aramis enables the analysis of the deformation, determination of failure mode of the tested specimens and limit displacement between edges of the interface.

Keywords: Digital Image Correlation System, concrete structures, measuring systems

1. INTRODUCTION

Modern measuring devices became more popular, one type of them is Digital Image Correlation system (DIC). That kind of tools gives new research capabilities. Using DIC during the test is much safer, because testing element is observed from a distance. It is important especially if sudden failure is expected. Besides indubitable advantages, using DIC is connected with limitations, which are

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specified in the paper. Measuring errors of strains under some conditions could represent an order of magnitude for ultimate concrete strain. Examples of experimental investigations of concrete structures conducted in Department of Concrete Structures of Lodz University of Technology [1÷3], by authors are presented in the paper. Discussion of obtained results can be used by other researchers to make interpretation of their experiments easier.

2. UNCERTAINTY ANALYSIS

Essential issue is to determine the uncertainties of measurements. It is very complex task, for analytical approach. *Kneć* [4] and *Revilock* at al. [5] keep a record of measurement noise then they deduced about inaccuracies. That method requires to take many photos of surface. It is assumed that unloaded specimen, not subjected to temperature changes should look identically in the each image. If any changes are observed they are classified as measurement noise. That procedure can be easily make and all impacts are considered. Very important is to keep constant light condition during the test.

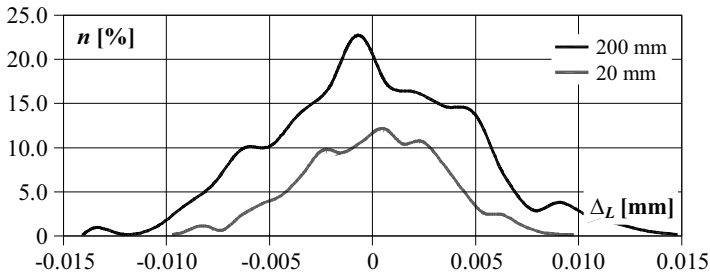


Fig. 1. Deviation of length of virtual measuring bases and frequency of its occurrence – base length 20 mm and 200 mm (unloaded specimen).

In order to determine inaccuracies 400 measurements (800 pictures) of surface have been made. Deviation value and frequency of its occurrence are presented in Fig. 1 (length) and in Fig. 2 (strain). The readings from -0.013 mm to $+0.013$ mm for 200 mm and from -0.009 mm to $+0.008$ mm for 20 mm base were recorded. Statistical analysis for that data were made. Standard errors were calculated. Their value are equal to 0.005 mm and 0.003 mm for longer and shorter base, respectively. These measurements can be considered as precise – the typical scale division for optic compactor is 0.05 mm.

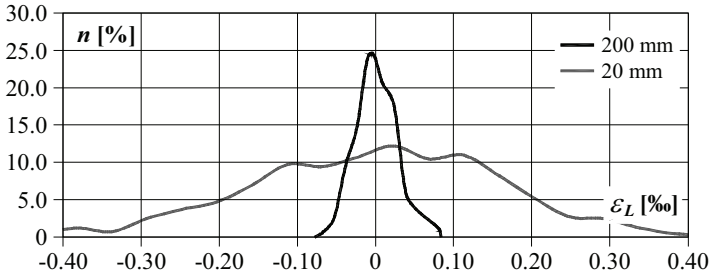


Fig. 2. Deviation of strains for linear measuring bases and frequency of its occurrence – base length 20 mm and 200 mm (unloaded specimen)

The linear strains can be also measured by using Digital Image Correlation. Noise values for strain are in range from -0.068% to 0.068% for 200 mm and from -0.441% to 0.363% for 20 mm base. The standard errors values are 0.024% and 0.150% . The longer base the smaller error is. Both values are much higher than error of strain gauges.

3. MEASUREMENTS OF CRACK WIDTH

Measuring crack width is possible by using Aramis system. However it is not easy to make it properly. Aramis creates maps where accumulation of strains can be find – regions where the cracks are occurring. The way of determining the crack width is presented in an example of corbel tested in Lodz. In the Fig. 3b measuring bases for crack 3 and 3' are presented. It was initially assumed that anchor points of measuring base should be outside of the strain accumulation area (crack 3). It is according to the method presented by *Goszczyńska* et al. [6] or *Goszczyńska* and *Tworzewska* [7]. Authors stated that discussed approach gives higher width of crack than values obtained using optical compactor (3 opt.) – see Fig. 3a. It was found that measurements carried out by using base (Crack 3') ensure good compatibility with measurement (3 opt.). In the presented example length of strain accumulation area is about 42 mm while the dimension of the (Crack 3') base is only 12 mm.

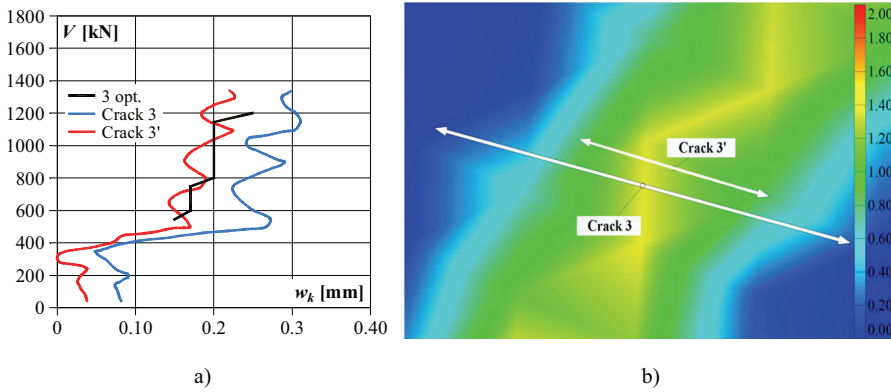


Fig. 3. Crack no. 3 a) comparison of crack widths measured traditionally (3 opt.) and by Aramis, b) measuring bases used by Aramis: (Crack 3) and (Crack 3')

For correct measurements of crack width reference length of base have to be found. It is easy to have correct results if some optical measurements for chosen load level are made. It is necessary to find length of base which gives values similar to reference value, then the rest of the results will be trustworthy.

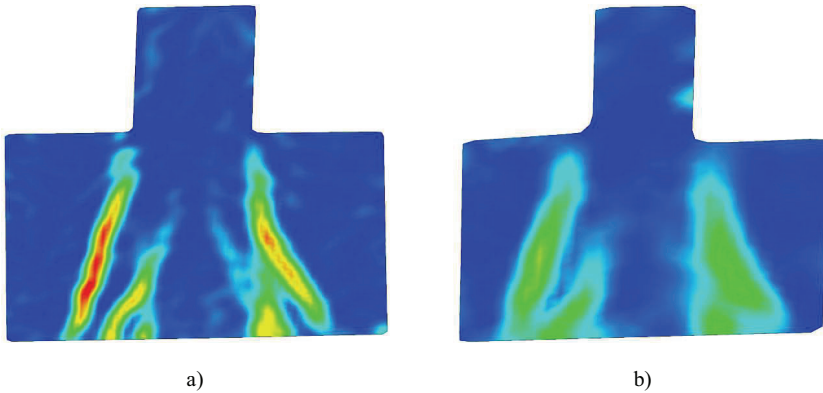


Fig. 4. Comparison of principal strain maps for corbel E – II at load equal to 1000 kN. Surface divided into facets a) 15×15 , b) 45×45

Measurements provided by using Digital Image Correlation are possible by dividing the test surface into fixed-size fields called facets. The standard facet features is 21×21 pixels. Based on the comparison between reference image and images recorded during the test it is possible to determine

the displacement of selected points on the surface. On this basis, the deformation can be calculated. The facet density affects the results in a manner analogous to the grid of the finite element method. To illustrate this phenomenon two different sizes of facets were considered: 15×15 and 45×45 pixels. The image of major deformation of the E-II corbel for the dense and thinning arrangement of facets is shown in Fig. 4. The deformation maps refer to the same element at a load level of 1300 kN. Comparing Fig. 4a and 4b, it can be stated that for larger facets, the features of the cracks were considerably more blurred and the deformation images were less detailed.

4. DIC IN THE TESTS OF SPECIMENS SUBJECTED TO SHEAR CUTTING

The investigations were intended to qualitatively assess the problem of shear cutting of concrete. This phenomenon can occur for elements with low shear slenderness $\lambda = a_c/h$ (where a_c is the distance between point of load and the edge of the support and h is the height of the element) such as very short reinforced concrete corbels. For this purpose, three "Z" shaped specimens made of the same concrete as a whole, were tested – see Fig. 5. The only variable parameter was the area of reinforcement transverse to the contact surface (A_{sw}). During loading the images were recorded by using cameras included in the Digital Image Correlation system Aramis (10 frames per second). The load was increased gradually at 30 kN, however at higher load level (about 60 ÷ 70% of ultimate load) the force was applied in uniform manner.

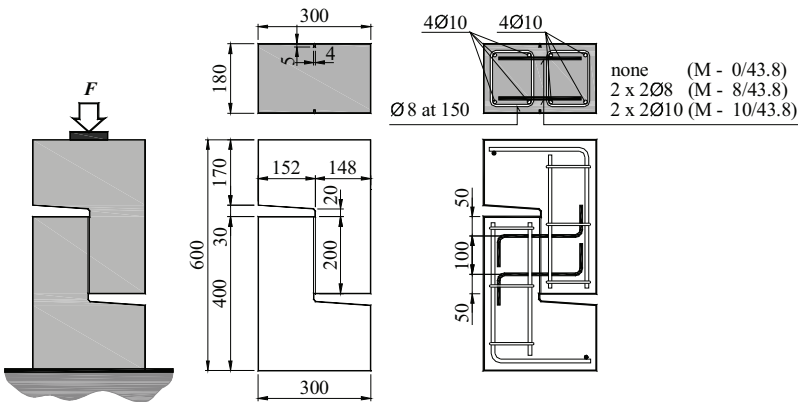


Fig. 5. Dimensions, shape and reinforcement of the test specimens

The use of the DIC system ARAMIS enabled for accurate registration of the destruction process in case of all models. The failure of M – 0/43.8 specimen with unreinforced interface was violent. The splitting of the element occurred within 0.1 s and was preceded by the appearance of a vertical crack, visible in Fig. 6.

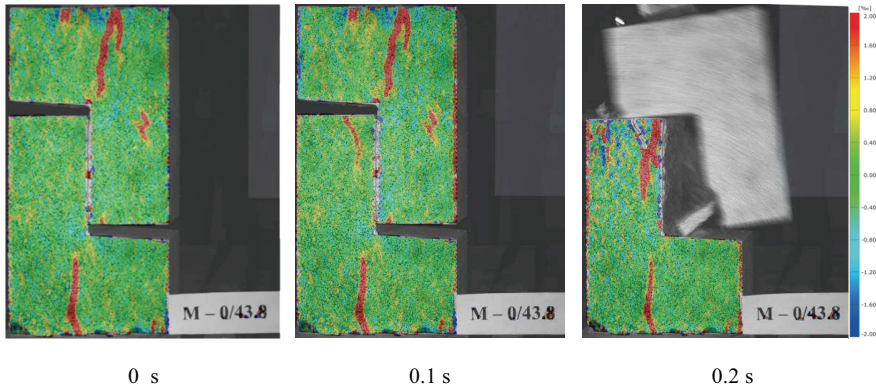


Fig. 6. Course of destruction of M – 0/43.8 specimen (at load of 198.6 kN)

Analysis of the strain maps based on recorded images allowed the qualitative assessment of the distribution of internal forces within the elements. Maps of deformation fields made possible to indicate the potential cracked zones and to determine the probable course of cracks. Comparing Fig. 7a and 7b with Fig. 7c a clear correlation between the zones of tensile strain accumulation (marked with red) and the cracks indicated during the tests can be seen. In the middle of the interface the compressive stress pass from one part of the element to the other. This is accompanied by the splitting forces visible on the maps of horizontal strains. Zones of tensile strain accumulation, visible in internal part of specimens, resulted from pulling as well as bending of transverse reinforcement (kinking effect).

The differences between the extent of the cumulative deformation zones and the course of the cracks marked during the test were due to the accuracy of human eye. The Digital Image Correlation allowed for much earlier prediction of zones of expected cracking.

Due to the lack of interface reinforcement and stress acting perpendicular to the contact surface, the load carrying capacity of the M – 0/43.8 specimen was dependent solely on the adhesion forces. Their disappearance was violent and occurred when the mutual movement of the edges of the interface was about 0.15 mm. Such displacement was about three times higher with respect to values recorded during the tests conducted by *Randl* and *Wicke* [8] who concerned interfaces between concrete cast at different times.

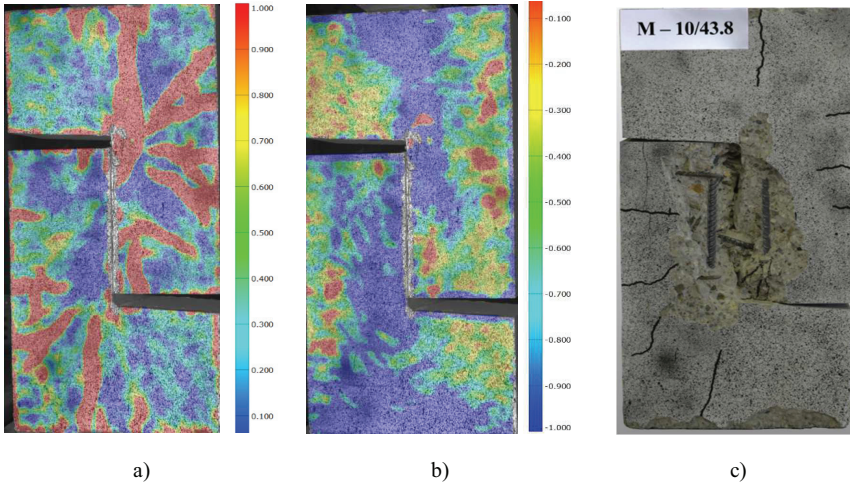


Fig. 7. Specimen M – 10/43.8: a) major strains, b) minor strains recorded before failure, c) front view after failure

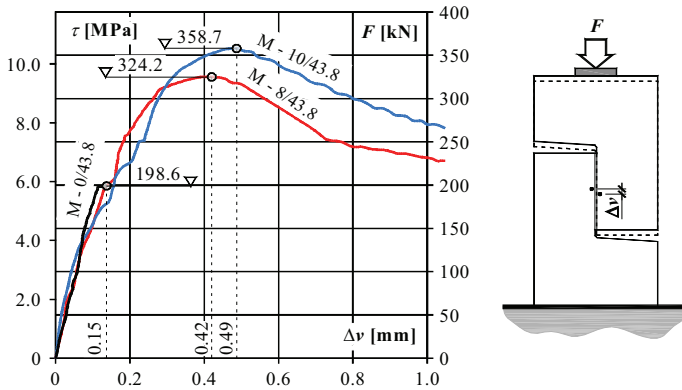


Fig. 8. Comparison of the mean displacements between edges of the interface

The introduction of transverse reinforcement made it possible to change the mechanism of interface failure – from brittle to ductile, and at the same time to increase the load capacity by 163% and 180% (M – 8/43.8 and M – 10/43.8 respectively) with respect to the reference element M – 0/43.8. It resulted from two additional mechanisms: aggregate interlock and dowel action that could not work in the case of unreinforced interface. Registered images allowed for accurate determination of the displacements associated with achieving the ultimate load – see Fig. 8.

At the further stage of the test, intense increase in deformation, which lead to rupture of the transverse reinforcement, was observed. The appearance of each “step” in the load – deformation curves was accompanied by a characteristic thwacks, hearable during the test. This allows to state that the measurements carried out by using the DIC system made possible to determinate accurately the moment of rupture of interface reinforcement.

5. CONCLUSIONS

According to results of the tests concerning corbels and “Z” shaped specimens many advantages of using Digital Image Correlation system can be seen. It enables to determine location of cracks just before the cracks have become visible by human eye, to estimate the width of the cracks as well as to follow the development of model deformation (eg. by using the virtual extensometers). The biggest advantage of using DIC is no need to install the necessary measuring devices before the tests. The analysis of recorded images can be carried out later, when the test is over. The virtual measuring equipment (eg. extensometers) can be placed wherever it is need. The use of the optical measuring device has allowed to record the deformations of M – 0/43.8 specimen even after the maximum load has been exceeded – without damaging of the transducers as would be in case of direct contact measuring methods.

However, it should be borne in mind that the results of measurements carried out by Digital Image Correlation systems can be flawed. Measurement errors are strongly dependent on the: size of the measuring area, inaccuracies of averaging the results of calculations, light condition changes during the test, noises of digital camera, angle between cameras and analyzed surface, quality of the lenses as well as preparation of the test surface.

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LIST OF FIGURES AND TABLES:

Fig. 1. Deviation of length of virtual measuring bases and frequency of its occurrence – base length 20 mm and 200 mm (unloaded specimen).

Rys. 1. Wartość rozrzutu długości wirtualnej bazy i częstotliwość ich występowania – baza o długości 20 mm i 200 mm (próbka nieobciążona)

Fig. 2. Deviation of strains for linear measuring bases and frequency of its occurrence – base length 20 mm and 200 mm (unloaded specimen)

Rys. 2. Wartość rozrzutu odkształceń dla pomiaru na bazie i częstotliwość ich występowania – baza o długości 20 mm i 200 mm (próbka nieobciążona)

Fig. 3. Crack no. 3 a) comparison of crack widths measured traditionally (3 opt.) and by Aramis, b) measuring bases used by Aramis: (Crack 3) and (Crack 3')

Rys. 3. Rysa numer 3 a) porównanie szerokości rozwarcia rysy mierzonej tradycyjnie (3 opt.) i za pomocą Aramisa, b) bazy pomiarowe użyte przez Aramisa: (Crack 3) i (Crack 3')

Fig. 4. Comparison of principal strain maps for corbel E – II at load equal to 1000 kN. Surface divided into facets a) 15×15 , b) 45×45

Rys. 4. Porównanie map odkształceń głównych wspornika E – II przy sile równej 1000 kN. Powierzchnia podzielona na fasetki a) 15×15 , b) 45×45

Fig. 5. Dimensions, shape and reinforcement of the test specimens

Rys. 5. Wymiary, kształt i zbrojenie badanych elementów

Fig. 6. Course of destruction of M – 0/43.8 specimen (at load of 198.6 kN)

Rys. 6. Przebieg zniszczenia elementu M – 0/43.8 (przy obciążeniu 198.6 kN)

Fig. 7. Specimen M – 10/43.8: a) major, b) minor strains recorded before failure, c) front view after failure

Rys. 7. Element M – 10/43.8: a) maksymalne, b) minimalne odkształcenia główne zarejestrowane przed zniszczeniem, c) widok po zniszczeniu

Fig. 8. Comparison of the mean displacements between edges of the interface

Rys. 8. Porównanie średnich przemieszczeń pomiędzy krawędziami płaszczyzny ścinania

SYSTEMY CYFROWEJ KORELACJI OBRAZU W BADANIACH EKSPERYMENTALNYCH: MOŻLIWOŚCI I OGRANICZENIA

Słowa kluczowe: systemy cyfrowej korelacji obrazu, konstrukcje żelbetowe, systemy pomiarowe

STRESZCZENIE:

Celem artykułu jest przedstawienie możliwości i ograniczeń stosowania systemów cyfrowej korelacji obrazu (CKO). W celu oceny niedokładności pomiaru przeanalizowano pole pomiarowe o wymiarach 1250×1100 mm, wykorzystując dwa aparaty o rozdzielczości matryc 6 megapikseli. Stwierdzono bardzo dobrą dokładność wyznaczenia zmiany długości odcinka. Dzięki temu obserwację szerokości rys można uważać za dokładną. W artykule zawarto praktyczne informacje dotyczące tego, w jaki sposób prowadzić pomiar szerokości rozwarcia rys za pomocą CKO. W drugiej części artykułu przedstawiono wyniki badań dotyczących nośności styku elementów betonowanych w tym samym czasie. Zastosowanie optycznego układu pomiarowego Aramis umożliwiło analizę odkształcenia, określenie sposobu zniszczenia badanych próbek i granicznych przemieszczeń między krawędziami połączenia.