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Combined Bundle Adjustment with Distances and GPS Observations**

1. Introduction

Today, automatic image orientation and automatic image-based dense reconstruction algorithms are often implemented in professional photogrammetric software. At the same time progress in computer vision brings many advantages to everyone who is dealing with image-based object modeling. A number of free, often on-line software for automatic sparse or dense reconstruction have appeared during the last years [8]. The majority of them allow only for fully automatic reconstruction [1, 7] which is good for amateurs but bad for specialists, as controlling some important aspects of processing, like camera calibration is not available. As a result generated models are out of scale and proper angular orientation, and cannot be used as metric products. Addressed problems have been overcome in professional photogrammetric software where the bundle adjustment with control points is a standard. However, control points collection is a time consuming process even for small objects. Besides it involves using additional survey equipment. As an alternative approach the direct geo-referencing methods seem to be a very attractive solution.

Nowadays a number of cameras are equipped with integrated GPS, or have the interface open for an external receiver. Smartphones and tablets as a standard are equipped with GPS, MEMS inertial sensors and cameras which still improve their resolution and image quality. Low-cost devices providing image direct geo-referencing could be potentially utilized to improve the accuracy of obtained photogrammetric products, bringing new functionality to free, image-based modelling software. The only problem with low-cost GPS receivers is the low accuracy of positioning. The positioning errors can be in the considerable extend compensated by the photogrammetric observations [2, 5] however not completely. GPS measurements of projection centers, when conducted in the network with good controllability, allow

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for the estimation of angular elements of external orientation using only low cost inertial sensors [6] or even without using inertial sensors at all [2]. Looking for cheap and simple approaches to improving accuracy in small-area combined image networks one may think about direct distance measurements. Ellum [3] tried to include single distance observations into a small terrestrial image network achieving some improvement in final accuracies. Kolecki [5], similarly to Ellum, didn't notice a considerable decrease of errors. However, both authors used survey-grade GNSS receivers in their systems. Kolecki [5] in his simulations proves that distance observations have the potential to improve accuracy of image orientation only if lower accuracy GPS is involved. In the case of using a survey-grade receiver, distance measurements are simply redundant. In this paper more experiments and simulations are addressed to give deep insight into the nature of distance observations and their impact on adjustment results.

2. Functional Model

The functional model of applied bundle adjustment consists of three kinds of equations. The majority of observations are the image coordinates, bringing into the model equations of collinearity (1) in its usual form. The observed external orientation parameters are included using equations (2) and (3). The distance observations are included by adding into the model equation (4):

$$x_i^k + v_{x_i^k} = x_0 - ck \frac{r_{11}^k (X_i - X_0^k) + r_{21}^k (Y_i - Y_0^k) + r_{31}^k (Z_i - Z_0^k)}{r_{13}^k (X_i - X_0^k) + r_{23}^k (Y_i - Y_0^k) + r_{33}^k (Z_i - Z_0^k)} + \Delta x_i^k \quad (1)$$

$$y_i^k + v_{y_i^k} = y_0 - ck \frac{r_{12}^k (X_i - X_0^k) + r_{22}^k (Y_i - Y_0^k) + r_{32}^k (Z_i - Z_0^k)}{r_{13}^k (X_i - X_0^k) + r_{23}^k (Y_i - Y_0^k) + r_{33}^k (Z_i - Z_0^k)} + \Delta y_i^k$$

$$\begin{aligned} X_0^{obs} + v_{X_0} &= X_0 \\ Y_0^{obs} + v_{Y_0} &= Y_0 \\ Z_0^{obs} + v_{Z_0} &= Z_0 \end{aligned} \quad (2)$$

$$\begin{aligned} \alpha^{obs} + v_{\alpha} &= \alpha \\ \mathbf{v}^{obs} + v_{\mathbf{v}} &= \mathbf{v} \\ \kappa^{obs} + v_{\kappa} &= \kappa \end{aligned} \quad (3)$$

$$d_{ij} + v_{d_{ij}} = \sqrt{(X_j - X_i)^2 + (Y_j - Y_i)^2 + (Z_j - Z_i)^2} \quad (4)$$

where: X_0^k , Y_0^k and Z_0^k stand for projection center coordinates, X_i , Y_i i Z_i are the object point coordinates, α , ν and κ are the angles of image orientation, the r coefficients come from the rotation matrix, ck , x_0 and y_0 are the internal orientation parameters. As given in the equation (4), the distances are always measured between object points that must also be marked in the images as tie points.

3. Testing

3.1. Data Preparation

The aim of this experiment is the evaluation of adjustment accuracy with and without distances in ‘ideal conditions’. This means that the true external orientation parameters and their accuracy are known. Besides the distance observations that are free of errors can be provided. However it should be noticed that the influence of distances may depend on the overall network configuration and the distance spatial distribution. The examined network is shown in Figure 1. It has a closed-loop configuration to provide good controllability of observations.

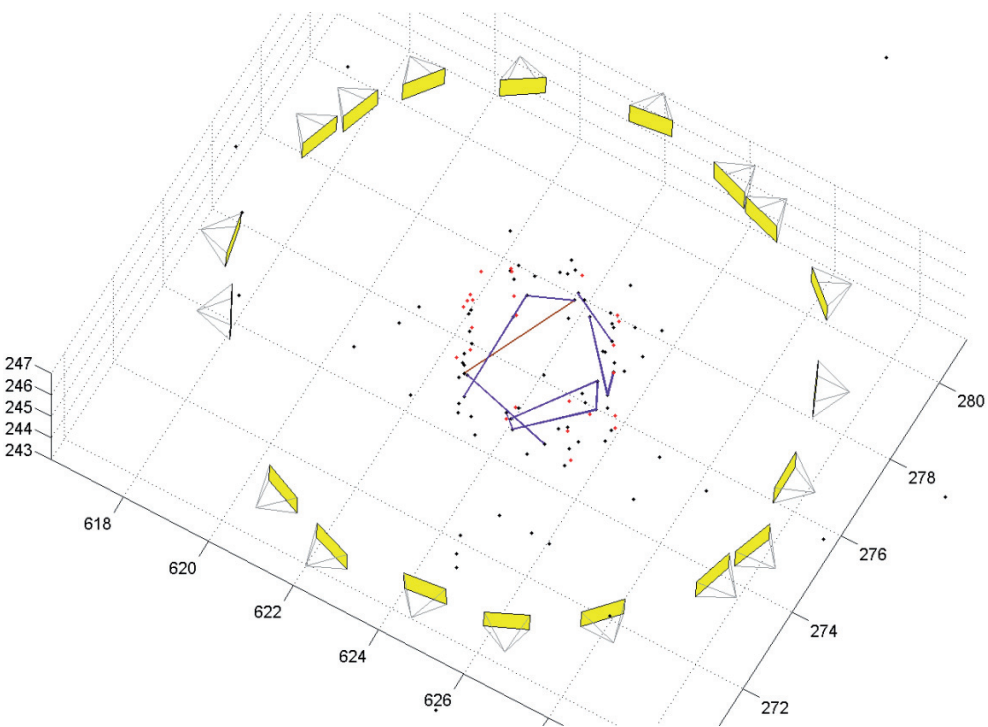


Fig. 1. Test image network with camera stations and distances. Tie and check points are marked in black and red respectively

During the preparatory works this network was adjusted with 10 control points, to calculate the “truth” external orientation parameters. Then the 11 distances were calculated using the adjusted coordinates of tie points. As it can be seen in Figure 1 these distances are uniformly distributed. However it should be clear that some of them may be hard to determine by a field survey. The adjusted projection centers (PC) coordinates can be treated as true values. In this experiment these values are corrupted by the zero-mean random Gaussian noise with the σ parameter changing from 5 to 300 mm in the intervals of 5 mm. To improve the reliability of the simulations 40 sequences of such a random number were generated for each projection center coordinate. This means that the network can be adjusted with the corrupted coordinates 40 times for each σ . The number of 40 seems to be a reasonable compromise between calculation time and the reliability of the results. Altogether, for all values of σ , there are 2,400 adjustments in each adjustment scenario. For each adjustment the standard deviations for check points are calculated and averaged within bins of 40. One should expect the averaged series of errors will form the ascending line in the σ over RMSE plot.

3.2. Simulation Scenarios

Having the data prepared, the simulations can be started. The following scenarios of adjustment are going to be carried out:

- A0: 2,400 adjustments without distance observations and with constant *a priori* error of projection center coordinates,
- B0: 2,400 adjustments without distance observations, with *a priori* errors of projection centers changing according to values of sigma in the random number series,
- A1: 2,400 adjustments with only one distance and constant *a priori* error of projection center coordinates,
- B1: 2,400 adjustments with only one distance and *a priori* errors consistent with the values of sigma.
- A11: 2,400 adjustments with 11 distances and constant *a priori* error of projection center coordinates,
- B11: 2,400 adjustments with 11 distances and *a priori* errors consistent with the values of sigma.

In scenarios A the *a priori* errors of PC coordinates are 4 cm, 4 cm and 5 cm respectively for X, Y and Z. The aforementioned values seem to be the reasonable estimation of the expected accuracy of the real-time kinematic GPS survey in the built-up areas. The highest possible value of σ in the B scenarios (i.e. 30 mm) corresponds to the accuracy that may be achieved using pos-processed, code-only GPS surveys provided the good satellite visibility.

4. Results

4.1. Accuracy

Plots showing the relative accuracies of bundle adjustment for all adjustment scenarios are presented in Figure 2 (scenarios A) and Figure 3 (scenarios B). It can be noticed (Fig. 2) that underestimation of *a priori* errors of geo-referencing results in systematic accuracy drop. Including distances helps to get slightly better results. However, the effect of using 11 distances instead of 1 seems to be surprisingly insignificant. To make the overall conclusion one can examine the 3D position error (Fig. 2d) and notice that including distances provides about 20–25% accuracy improvement. However it is clear that this rule may not hold in networks with different image number and different geometry.

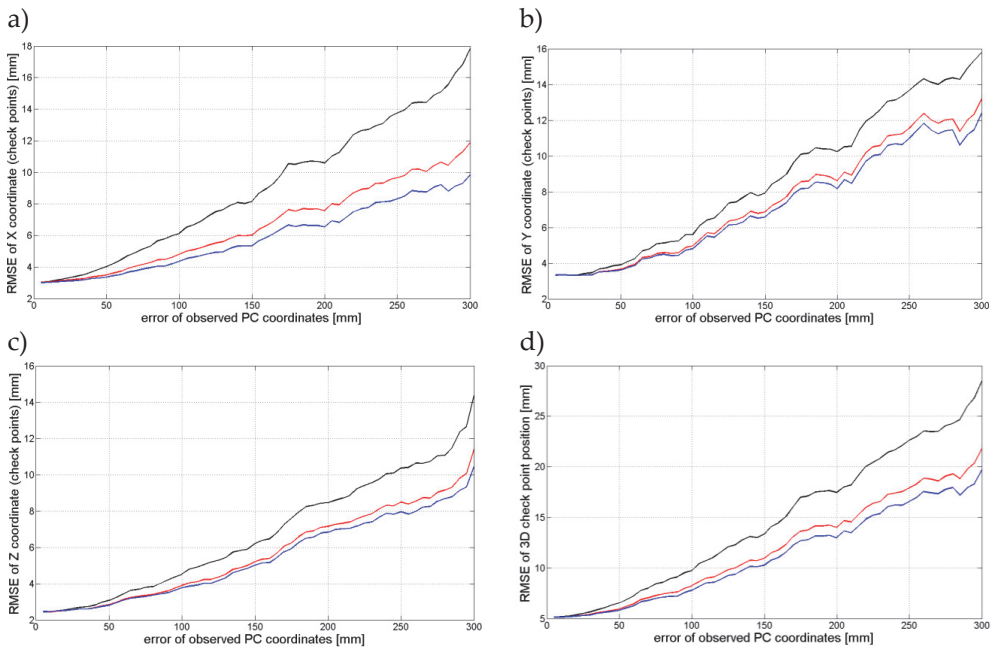


Fig. 2. Relative accuracy of bundle adjustment with constant errors of PC coordinates in scenario A; black: no distances, red: one distance, blue: eleven distances

In practice such considerable under (or over) estimation of *a priori* errors is not likely to happen, but the experiment shows some general phenomena, and illustrates the importance of assessment of observations accuracy in the context of adjustment. The extent in which *a priori* accuracy is over estimated is illustrated in Figure 4, where the change of standard deviation of an observation of unit weight is shown.

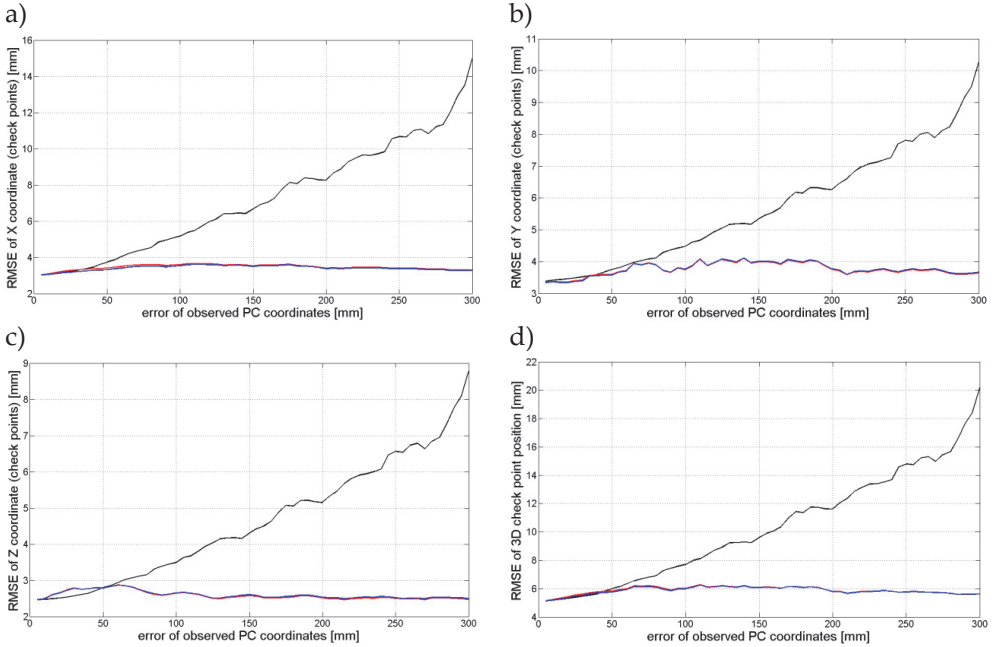


Fig. 3. Relative accuracy of bundle adjustment with truth *a priori* errors of PC coordinates in scenario B; black: no distances, red: one distance, blue: eleven distances

When the weights in adjustment process are consistent with the true accuracies of PC coordinates, the distance observations stop the check point errors from increasing (Fig. 3). Some remaining errors are still present (about 3 mm for each coordinate) possibly due to initial inaccuracy of check points, nevertheless the accuracy seems not to decrease, even though the PC errors reach 30 cm. However if the distances were excluded from the adjustment, the check point errors would increase noticeably (black lines in Figure 3c, d). What is more interesting, the number of observed distances has no influence on accuracy. The blue and red lines in the plots shown in Figure 3 almost coincide. It seems that in the assumed conditions of simulation of scenario B (true EO and true distances consistent with the observations, true *a priori* accuracies are known), observed distances do not help to correct the network geometry locally. Instead the distance observations correct the scale of the network, which in the case of no distances, would be disturbed by the erroneous observed PC coordinates. The rotation of the network may not be disturbed sufficiently to cause significant check point errors. In scenario A the value of the standard deviation of unit weight is not influenced by increasing the errors of the observed PC coordinates (Fig. 4). However its value is lower than 1.0 due to the overestimated errors of the image coordinates of the tie points.

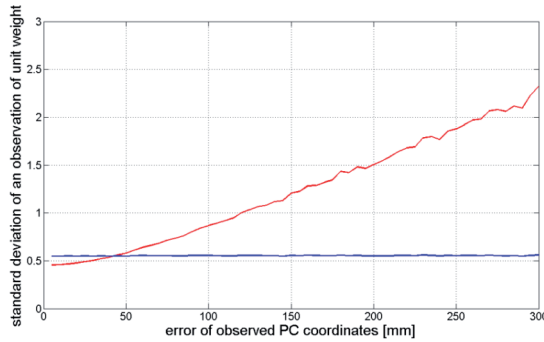


Fig. 4. Dependency of the standard deviation of an observation of unit weight from the true error of observed PC coordinates; red – fixed *a priori* errors of PC coordinates, blue – weights of observed PC coordinates consistent with true errors

4.2. Scale

Further calculations were carried out in order to show the role the observed distance plays in the bundle adjustment of the test network, especially its influence on the scale. For scenarios B0 and B1 the 3D 7 degrees-of-freedom transformation was calculated according to the Horn closed-form solution [4]. Scale checking was conducted for 15 networks in scenario B0 and for 15 networks in scenario B1. For each scenario checked networks were grouped in 3 bins, each containing 5 networks with the same errors of PC coordinates. The values of the PC coordinate error and corresponding average scale is given in Table 1. According to the terminology of Horn, as the right (target) data set the true check point coordinates were utilized. For scenario B1 the deviation of the scale from the unity is kept close to the level of 10^{-4} irrespective of PC error. If the distance observation is removed (scenario B0) this deviation increases reaching the level of 10^{-3} for the 15 cm error of PC coordinates and 10^{-2} for the 30 cm error. Provided results show that using the distance observation can help to fix the scale of the network, which may be disturbed to a large extent by erroneous direct geo-referencing.

Table 1. Results of scale parameter estimation for networks with three different error levels of PC coordinates adjusted within scenarios B0 and B1

	B0			B1		
PC coordinates error [mm]	5	150	300	5	150	300
Average deviation from unity (absolute values) [unitless]	0.00013	0.00199	0.01050	0.00014	0.00012	0.00012

4.3. Redundancy

Figure 5 allows further examination of the role of the observed distances in the adjustment. If the constant weights are applied to PC coordinates, the redundancy of observed distances (one distance in case of Figure 5a) are also constant (Fig. 5 – red line).

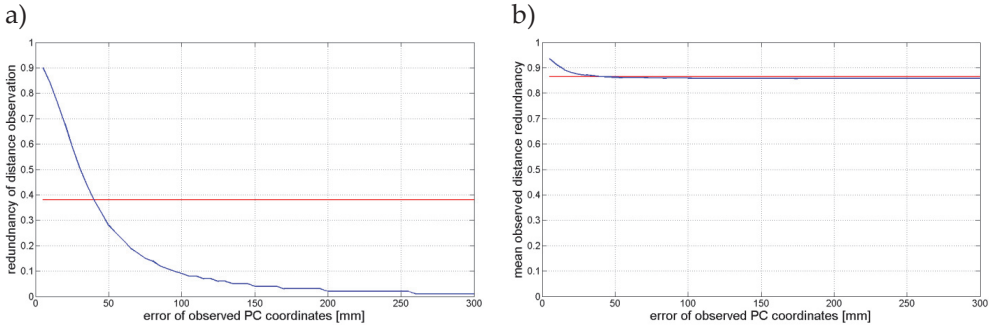


Fig. 5. Dependency of mean observed distance redundancy from the true error of observed PC coordinates: a) scenarios with one distance (A1, B1); b) scenario with 11 distances (A11, B11); red – fixed *a priori* errors of PC coordinates, blue – weights of observed PC coordinates consistent with true errors

In Figure 5a it can be observed that the percentage of observation that is utilized in adjustment (complement of redundancy to the value of 1.0), in the scenario B1 increases from 10% to 99%. It means that for large PC coordinate errors, almost 100% of this single distance observation is utilized in the adjustment and the proper scaling of the network is almost fully dependent on the accuracy of this measurement. In the Figure 5b the mean redundancy has values closer to unity, as the total distance redundancy is distributed to single observations. The controllability of each distance is therefore better. What is quite interesting, in Figure 5a and in Figure 5b the blue line crosses the red line around the abscissa 40 mm – the value of PC X and Y coordinate error assumed for scenarios A.

5. Conclusions

The experiments presented in this paper show some phenomena that occur in the adjustment of distance-supported photogrammetric networks with direct geo-referenced images. Presented simulations allow for examining the interaction that takes place between observed projection center coordinates and distances for the image network with favorable geometrical configuration (closed loop) and good controllability. In that case the distance observations are error-free, they can play a great role in stabilizing the scale of network, that in case of erroneous geo-referencing can be easily disturbed. However one very important aspect should be kept in

mind, namely the proper weighting of the observed PC coordinates (and potentially angles). Underestimation of *a priori* errors may limit the corrective role of the distances and increase adjustment errors. Proper weighting of certain groups of observations may be achieved using the methods proposed by Ziobro [9, 10]

In practice photogrammetric networks may not have as good controlability as network presented in this paper (Fig. 1). Besides the true accuracy of PC coordinates remains unknown and the accuracy of measured distances is limited. Often distances cannot be measured in versatile directions like those in scenario B11. Similar tests, also for networks consisted of strips, are presented by Kolecki [5]. However, in those simulations the true PC coordinate errors were unknown and the distances were measured between terrain points with limited accuracy. Under those conditions distances also helped to improve accuracy, however not in such a large extent as in the presented tests.

Photogrammetric processing, especially the close range measurements, can be supported by additional information about objects. This may include not only distances but also tie lines and imposed constraints like vertical, horizontal, parallel and orthogonal directions. Also, measurements with geo-referencing sensors should help improve the accuracy of created models opening fully-automated photogrammetry to many applications.

References

- [1] ARC3D. [on-line:] <http://www.arc3d.be/> [access: 17.12.2013].
- [2] Bartelsen J., Mayer H.: *Orientation of Image Sequences Acquired from UAVS and with GPS Cameras*. International Calibration and Orientation Workshop EuroCOW 2010, Castelldefels, Spain, 2010.
- [3] Ellum C.M., El-Sheimy N.: *Portable Mobile Mapping*. International Conference – New Technology for a New Century, FIG Working Week 2001, Seoul, South Korea, 2001 [on CD-ROM].
- [4] Horn B.K.P.: *Closed-form solution of absolute orientation using unit quaternions*. Journal of the Optical Society of America, vol. 4 (4), 1987, pp. 629–642.
- [5] Kolecki J.: *Wyznaczenie elementów orientacji zewnętrznej zdjęć naziemnych z wykorzystaniem obserwacji fotogrametrycznych i inercyjnych oraz satelitarne go systemu pozycjonowania*. Wydawnictwa AGH, Kraków 2013.
- [6] Kolecki J., Kuras P.: *Reliability of Terrestrial Image Geo-referencing using GPS and MEMS inertial measurement unit*. International Calibration and Orientation Workshop EuroCOW 2012, Castelldefels, Spain, 2012.
- [7] My3DScanner. [on-line:] <http://www.my3dscanner.com/> [access: 17.12.2013].
- [8] Snavely N., Seitz S.M., Szeliski R.: *Modeling the World from Internet Photo Collections*. International Journal of Computer Vision, vol. 80 (2), 2008, pp. 189–210.

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- [9] Ziobro J.: *Przedwyrównawcze wykrywanie błędów grubych w pomiarze środków rzutów dla aerotriangulacji*. Archiwum Fotogrametrii, Kartografii i Teledetekcji, vol. 16, 2006, pp. 609–618.
- [10] Ziobro J.: *Przedwyrównawcze wykrywanie błędów grubych w pomiarze kątów orientacji dla aerotriangulacji*. Archiwum Fotogrametrii, Kartografii i Teledetekcji, vol. 24, 2012, pp. 421–413.